GE
159 Plastics Avenue
Pittsfield, MA 01201
USA

Transmitted Via Electronic Mail/Overnight Delivery

August 25, 2006

Ms. Sharon Hayes
United States Environmental Protection Agency
EPA New England
One Congress Street, Suite 1100
Boston, MA 02114-2023

Re: GE-Pittsfield/Housatonic River Site
Hill 78 and Building 71 On-Plant Consolidation Areas (GECD210 & GECD220)
August 2006 Addendum to Phase II Final OPCA Cover Construction Plan

Dear Ms. Hayes:

This letter addresses several conditions identified by the U.S. Environmental Protection Agency (EPA) in an August 1, 2006 letter conditionally approving the General Electric Company's (GE's) May 5, 2006 submittal regarding the Hill 78 and Building 71 On-Plant Consolidation Areas (OPCAs), entitled *2006 Consolidation and Phase II Final Cover Construction*. That GE submittal described the anticipated 2006 consolidation activities at the OPCAs and the final cover installation activities proposed for the remaining areas of the OPCAs. This letter addresses the specific EPA comments contained in its August 1, 2006 letter and, as such, serves as an addendum to GE's May 5, 2006 submittal. In addition, based on other recent communications between EPA and GE, this letter includes additional information related to certain OPCA consolidation and cover construction activities. The information in this letter supplements, and in some cases modifies, the information presented in GE's May 5, 2006 submittal.

The contents of this letter are organized to follow the format of EPA's August 1, 2006 letter; a GE response is provided for each EPA comment. In addition, an attachment to this letter (Attachment A) provides additional design information requested by EPA.

EPA Comment 1:

During final cover installation, GE shall continuously evaluate site conditions (e.g., high winds, high temperatures, humidity extremes, high air monitoring results, visible dust, etc.) and specific earthwork activities (e.g., placement of find sands, etc.) that may result in uncontrolled vapors/visible dust and possible exceedances of particulate and PCB action levels. If necessary, GE shall propose additional operational procedures to be implemented as needed, above and beyond those required by condition #1 above and those described in the EPA-approved June 1999 Detailed Work Plan for On-Plant Consolidation Areas and the April 2006 Addendum to OPCA Work Plan, to ensure that final covers are installed in a controlled manner without undue delays.

GE Response:

Comment noted.

* * *

EPA Comment 2:

GE shall ensure that, prior to placement of the final cover material, the 3- to 6-inch thick layer of wood chips used for daily cover is mixed with the waste material to minimize the potential of distinct, continuous layers of wood chips remaining within the consolidation area.

GE Response:

Agreed. Currently, wood chips are used for daily cover only at the Hill 78 OPCA. The wood chips are removed to the extent practicable prior to the placement of subsequent consolidation materials. The wood chips are then reused and eventually become mixed with the consolidation materials. However, prior to placement of the GCL or FML components of the final cover, the wood chips used for daily cover will be mixed with the underlying consolidation materials (or with imported soil materials that may be used to prepare the final subgrade surface).

* * *

EPA Comment 3:

GE shall provide daily operational procedures and sequencing including placement of daily cover over the area of the cap under construction for all layers (for example, will GE uncover, grade, and recover one-acre sized areas, or less, on a daily basis and, if so, with what material?).

GE Response:

In light of the remaining available weeks in the 2006 construction season, the OPCA area anticipated to be capped in 2006 consists only of the remaining portion of the Building 71 OPCA (approximately 2.8 acres in size). The approximate 1.9-acre portion of the Hill 78 OPCA originally anticipated to be capped in 2006 (as indicated in GE's May 5th submittal) will not be subject to final cover installation this year. Based on discussions with EPA, the installation of a final cover over the remaining portion of the Building 71 OPCA will generally be performed in three phases: 1) subgrade preparation of the entire 2.8 acres (approximately one acre at a time); 2) geosynthetics installation over the entire 2.8 acres; and 3) soil cover placement and hydroseeding in 0.5-acre increments. The general sequence of operational procedures anticipated to be performed by the remediation contractor during each phase of final cover installation is as follows:

- A portion of the OPCA subgrade surface will be uncovered (i.e., the daily cover will be removed) at the start of each construction day (7 AM or later, as further discussed below). The uncovered area of the subgrade surface will not exceed approximately one acre at any given time.

- The subgrade surface will be prepared by fine grading and removing any deleterious materials on the surface that could impact the overlying geosynthetics. Where necessary, a thin layer of imported soil

may be used to form a smooth uniform surface. Following the grading activities, the surface will be subject to smooth-drum rolling. If the overlying geosynthetics are not available for immediate placement on the prepared subgrade, the area will be tarped, and the next portion of subgrade (not to exceed approximately one acre) will be uncovered and prepared as discussed above.

- Consistent with the previously approved cover plan, the top "plateau" areas of the OPCAs will be covered with geosynthetic clay liner (GCL). Only the amount of GCL that can be subsequently covered by flexible membrane liner (FML) that same day will be installed. For other areas (i.e., along the steeper slopes), the GCL will not be installed; rather, FML will be placed directly over the prepared subgrade.

- FML and an overlying geosynthetic drainage composite (GDC) will then be installed over the entire area of prepared subgrade, including over the GCL (up to approximately one acre). Any area of prepared subgrade that is not covered by the FML at the end of each day will be re-tarped.

- As the FML and GDC are deployed and installed, additional areas of subgrade will be uncovered and prepared (if necessary) concurrently. FML and GDC deployment will continue over the newly prepared subgrade until the entire area identified for final cover installation is covered with FML and GDC. For 2006, as noted above, it is anticipated that FML and GDC will be installed over the entire remaining 2.8 acres of the Building 71 OPCA that have not been previously covered with the final cover system. Again, however, at no time will the area of exposed subgrade exceed approximately one acre (i.e., not be covered by tarps or FML).

- Once the entire area identified for final cover installation is covered with FML and GDC, overlying soil layers will be installed. The soil protection layer will be installed and compacted in a single 18-inch-thick layer over an area not to exceed 0.5 acre. Once 0.5 acre of the FML and GDC is covered with the 18-inch-thick soil cover layer, a 6-inch-thick layer of topsoil will be installed. Once 0.5 acre of topsoil is installed, the entire 0.5 acre will be hydroseeded with seed, fertilizer, mulch and tactifier. (Depending on weather conditions, a sprinkler system may be needed until there is sufficient vegetative growth to eliminate dust generation.). The next section of FML and GDC (0.5 acre maximum) will then be covered with the 18-inch-thick soil cover layer.

- Once final grading of the subgrade surface is initiated, as well as during installation of the soil components within the final cover system, daily covers will be used at the end of each day, consisting of tarps (for subgrade, soil cover, or topsoil), hydromulch (for soil cover or topsoil only), or in the case where the completed topsoil layer is the final exposed surface, hydroseed. Spraying of the subgrade material and cover soils with water during their placement (until daily cover is installed) will also be performed to further control dust generation. At the start of the following work day, the tarps will be removed to the extent necessary for completion of the subgrade surface or a particular soil layer; if used for daily cover, the hydromulch will be covered with the remaining soil layer(s).

- The above cover construction activities (including re-tarping and other end-of-day housekeeping activities) will be performed on weekdays between the working hours of 7 AM and 6 PM. There will be no equipment start-up before 7 AM, and material transport vehicles (including those vehicles transporting soil cover materials) will not enter the site after 4 PM. Under some circumstances, it may be necessary to perform certain specific, time-critical activities (e.g., geosynthetics installation) after 6 PM. In those instances, GE will request EPA approval (at least three days in advance) to

complete such activities after 6 PM. At no time, however, will on-site work activities extend beyond 7 PM, or be performed on weekends or federal holidays.

- Dust control measures will be performed continuously any time any subgrade or soil cover is exposed (i.e., not covered with tarps, hydromulch, or hydroseed).

- Air monitoring for airborne particulates will be conducted at the five OPCA air monitoring locations daily during the final cover installation activities (as well as during active consolidation activities), and monitoring for airborne PCBs will be performed at these five locations twice a week. Twice-weekly PCB air monitoring will continue through September 29, 2006, at which time the frequency will be re-evaluated by GE and EPA based on the anticipated activities at each OPCA.

As recently discussed with EPA, there may be periods of time when separate OPCA consolidation and cover construction activities will be occurring simultaneously. In this event, GE will ensure that all of the site control, dust control, and other precautionary measures that have previously been approved by EPA will be fully maintained for all areas subject to the ongoing activities (including maintaining separate work crews for each activity). For example, in this situation, multiple dust suppression activities will be implemented as necessary.

Lastly, GE is anticipating that if the proposed 2006 final cover installation activities are initiated in August 2006, they will be substantially completed by the end of 2006 (weather permitting). However, similar to the 2005 final cover installation activities, certain final cover features (e.g., drainage swales) may be completed during the following spring.

* * *

EPA Comment 4:

GE shall define "Low Permeability General Fill" in Detail 1 of Drawing 8.

GE Response:

The "Low Permeability General Fill" identified in Detail 1 of Drawing 8 was specified to preclude the use of high permeability soil (e.g., sand) during construction of the mid-slope swales. Until vegetation is established within the swale, lower permeability fill within the berm will promote stormwater flow within the swale and inhibit percolation into the berm. This material is defined as material that has a significant amount (at least 10%) of fine-grained material to limits its permeability. The required gradation characteristics of the soil material are indicated below.

Sieve Size	Percent Passing (%)
3"	100
#4	65-90
#40	50-80
#200 (indicating fine-grained material)	10-40
Clay-sized fraction to be 3% minimum	

* * *

EPA Comment 5:

GE shall identify where Detail 2 of Drawing 8 is used.

GE Response:

Detail 2 on Drawing 8 is a cross section (i.e., looking down the slope of the OPCAs) of a temporary final cover termination along the slope of the OPCA. This particular detail would be applicable to slope areas that will be partially covered with the final cover system, thereby leaving a temporary cover edge parallel to the slope. This detail illustrates how the edge of the final cover should be constructed for a future final cover connection. The pipe depicted on the detail is the HDPE pipe in the anchor trench located at the bottom of the slope.

* * *

EPA Comment 6:

GE shall identify the length of the 2H:1V slope in Detail 1 of Drawing 9 and clarify whether veneer stability during construction was considered. GE shall assess the feasibility of placing cover soil in a generally upslope direction (from the toe to the top of the slope) given the limited access to the toe of the sideslopes. In order to minimize tension, cover soil shall not be pushed from the top of the sideslopes down to the toe over geosynthetic materials.

GE Response:

The length of the temporary 2H:1V slope in Detail 1 on Drawing 9 is approximately 40 feet. This slope was not specifically analyzed for veneer stability during the design since it is a short-term, temporary condition that affects a relatively small portion of the slope (as shown on Drawing 2). The steeper slope in this area will be filled to 3H:1V during construction of the final cover system and retaining wall. Filling of this area will be performed from the bottom of the slope to the top to minimize tensioning of the underlying geosynthetics.

* * *

EPA Comment 7:

GE shall submit the design calculations for the retaining wall identified in Detail 2 of Drawing 9.

GE Response:

The design calculations for the retaining wall were prepared in November, 2002 by Associated Design Partners. Inc., a subcontractor to the retaining wall installer. The design calculations are included as Attachment A to this letter.

* * *

We trust the above responses are sufficient to address EPA's comments and that final cover installation activities can be initiated later in August. If you have any further questions, please feel free to contact me.

Sincerely,

W. A. Parker for/

Richard W. Gates
Remediation Project Manager

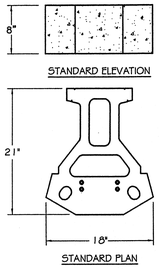
WAR/jlc
Attachments

cc: Dean Tagliaferro, EPA
Tim Conway, EPA
John Kilborn, EPA
Holly Inglis, EPA
Rose Howell, EPA*
K.C. Mitkevicius, USACE
Susan Steenstrup, MDEP (2 copies)
Anna Symington, MDEP*
Jane Rothchild, MDEP*
Linda Palmieri, Weston (2 copies)
Nancy E. Harper, MA AG*
Dale Young, MA EOEA*
Tom Hickey, Director, PEDA
Mayor James Ruberto, City of Pittsfield
Pittsfield Department of Health
Jeffrey Bernstein, Bernstein, Cushner & Kimmell
Teresa Bowers, Gradient
Michael Carroll, GE*
Andrew Silfer, GE
John Novotny, GE
Roderic McLaren, GE*
James Nuss, BBL
James Bieke, Goodwin Procter
Larry Kirsch, Goodwin Procter
Public Information Repositories
GE Internal Repository

**cover letter only*

Attachment A

Retaining Wall Engineering Drawings



STANDARD ELEVATION

STANDARD PLAN

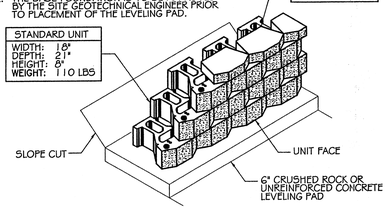
STANDARD UNIT
NOT TO SCALE

BASE LEVELING PAD NOTES:

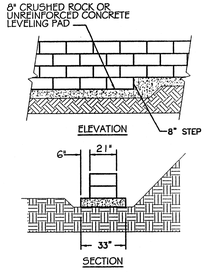
1. THE LEVELING PAD IS TO BE CONSTRUCTED OF CRUSHED STONE OR 15 MPA UNREINFORCED CONCRETE.
2. THE BASE FOUNDATION IS TO BE APPROVED BY THE SITE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF THE LEVELING PAD.

STANDARD UNIT
WIDTH: 18"
DEPTH: 21"
HEIGHT: 8"
WEIGHT: 110 LBS

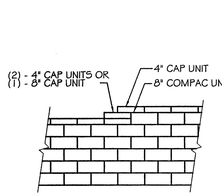
MINI CAP UNIT
WIDTH: 18"
DEPTH: 10"
HEIGHT: 4"
WEIGHT: 36 LBS



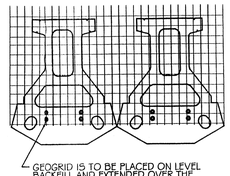
STANDARD UNIT/BASE PAD ISOMETRIC VIEW
NOT TO SCALE



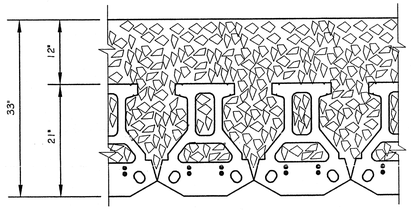
LEVELING PAD DETAIL
NOT TO SCALE



TOP OF WALL STEPS
NOT TO SCALE



GRID & PIN CONNECTION
NOT TO SCALE

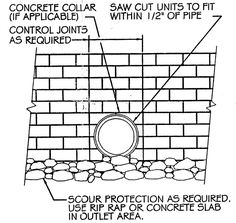


TYPICAL PLAN VIEW @ BLOCK
NOT TO SCALE

NOTE:
CRUSHED STONE UNIT COMPACT WITH HAND OPERATED PLATE COMPACTOR

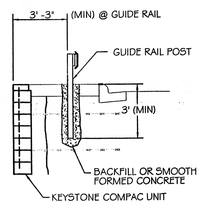
FILL ALL VOIDS INCLUDING PIN HOLES AFTER BLOCKS ARE ALIGNED. ALIGN EACH BLOCK COARSE PRIOR TO PLACING ADDITIONAL COARSE.

NOTE:
1. FOR PIPES LARGER THAN 24", A CONCRETE COLLAR MAY BE CAST AROUND PIPE FOR EASE OF CONSTRUCTION AND APPEARANCE.



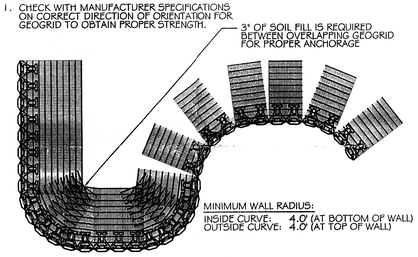
TYPICAL PIPE OUTLET DETAIL
NOT TO SCALE

NOTE:
1. AUGER THROUGH GEOGRID LAYERS.
2. BACKFILL OR CONCRETE GUARD RAIL POST IN PLACE

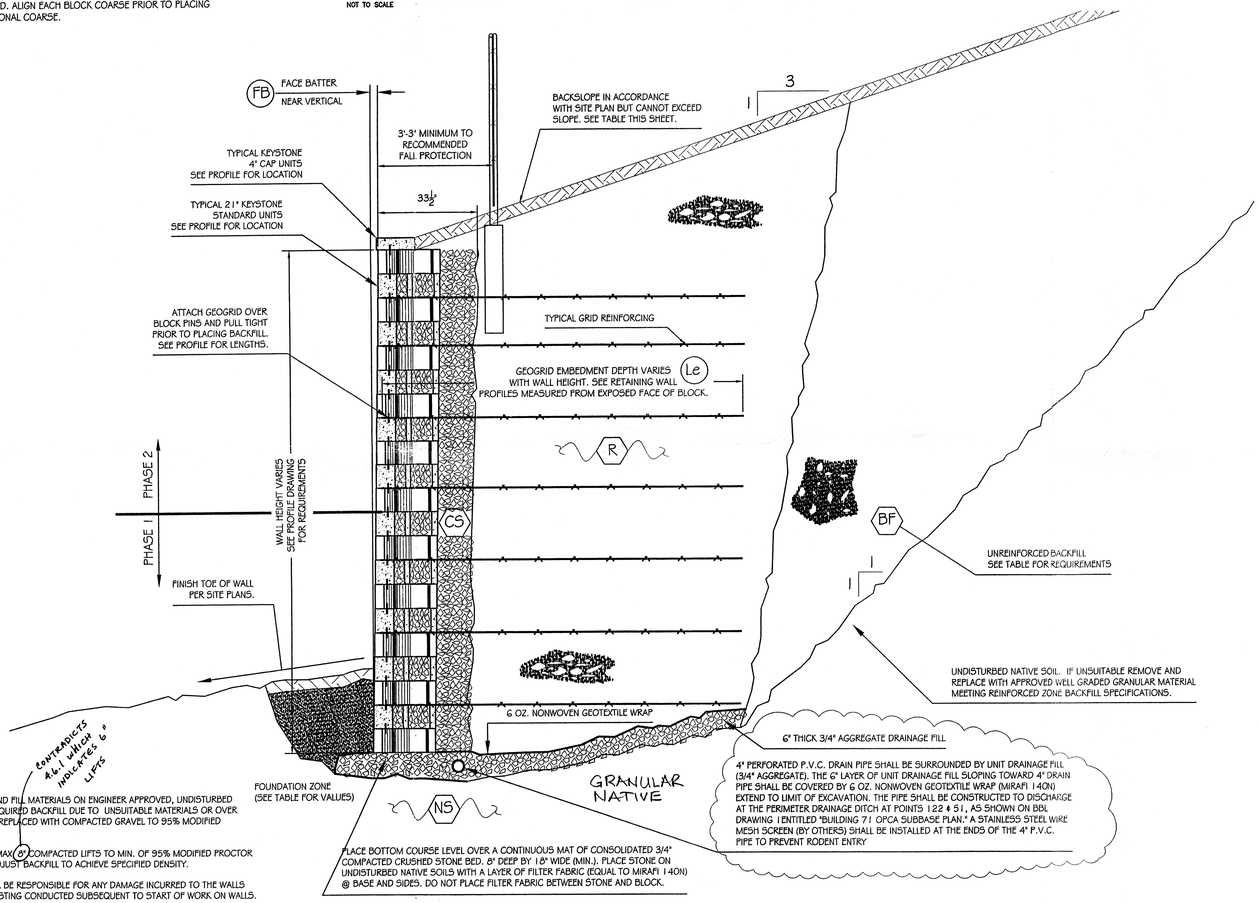


TYPICAL GUIDE RAIL DETAIL
NOT TO SCALE

NOTE:
1. CHECK WITH MANUFACTURER SPECIFICATIONS ON CORRECT DIRECTION OF ORIENTATION FOR GEOGRID TO OBTAIN PROPER STRENGTH.



GEOGRID INSTALLATION ON CURVES
NOT TO SCALE



NOTES:

1. CONSTRUCT WALL AND FILL MATERIALS ON ENGINEER APPROVED, UNDISTURBED NATIVE SOILS. ANY REQUIRED BACKFILL DUE TO UNSUITABLE MATERIALS OR OVER EXCAVATIONS WILL BE REPLACED WITH COMPACTED GRAVEL TO 95% MODIFIED PROCTOR DENSITY.

2. PLACE BACKFILL IN MAX. 8" COMPACTED LIFTS TO MIN. OF 95% MODIFIED PROCTOR DENSITY. MOISTURE ADJUST BACKFILL TO ACHIEVE SPECIFIED DENSITY.

3. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE INCURRED TO THE WALLS FROM ANY ONSITE BLASTING CONDUCTED SUBSEQUENT TO START OF WORK ON WALLS.

TYPICAL KEYSTONE WALL SECTION
SCALE: NTS

1. SHOP DRAWING ACTION STAMP:

1.1. This is an engineered shop drawing design based upon information provided by others. This plan requires review and approval of the site designer or otherwise responsible entity to ensure compliance with and acceptability of the assumed or represented design parameters. Unless specifically otherwise stated or represented review and acceptance of this shop drawing does not constitute acceptance of the specific internal stability design provided or represented here, it merely confirms the accuracy of information provided by others.(See note #1.2)

1.2. This design is based upon information provided by others. Should variations be encountered the contractor, site designer of record, or owner must notify the owner/engineer and Associated Design Partners, Inc, (ADP) to make appropriate adjustments.

2. SOIL REINFORCED WALL DESIGN NOTES:

2.1. External global stability calculations are typically provided by the owner's site, civil, or geotechnical engineer of record. External global stability has not been addressed as part of this internal stability wall design. Owner's site, civil, or geotechnical engineer shall assume responsibility of global stability.
2.2. Soil design values for this project were used based upon the site geotechnical study conducted by others or were assumed based upon observed or anticipated subsurface conditions. A summary of design values is included in the design values table. The site geotechnical engineer or site designer is required to review these values and confirm project compliance.

3. SOIL REQUIREMENTS:

3.1. Unreinforced backfill – Use granular, well graded, non–cohesive, free draining, soils with properties, as indicated in the design values table.
3.2. Reinforced fill material – Imported well graded gravel, free draining, and angular particles (submit Gradation and proctor for engineer's approval prior to start of construction) with properties, in accordance with the design values table. Well graded angular crushed gravel will normally comlpy with this specification.
3.3. Compaction – All soil fill shall be compacted to 95% of the maximum dry density in accordance with ASTM D–1557, modified proctor.
3.4. Crushed Stone – 3/4" angular crushed stone equal to MDOT 703.2 Stone. Rounded rock or pea stone is specifically not allowed.

4. GENERAL:

4.1. Place Keystone blocks on a 8" deep base footing of consolidated 3/4" crushed stone. Lean concrete may also be used for the base footing. If lean concrete is used it shall be placed on 6" of crushed stone, with smooth formed vertical surfaces, below anticipated frost depth.
4.2. Lock geogrid into blocks by hooking grid material over the fiberglass pins in the keystone block.
4.3. Install geogrid reinforcing fabric at locations and elevations shown on the profile elevation drawings.
4.4. Roll geogrid out with strong fiber (machine direction) direction perpendicular to wall face to embedment length (Le) as specifed on the profile elevations. Grid must be laid smooth, free of wrinkles, and pulled taut prior to fill placement.
4.5. Place a minimum of 6" of soil over grid before allowing machinery on the reinforcement area.
4.6. General soil compaction guidelines: Site excavation contractor is responsible for the methods and results of the compaction process. The following is a suggested method of installation.
4.6.1. Place soil in maximum 6" loose lift thickness and compact to 95% of the maximum dry density in accordance with ASTM D–1557, Modified Proctor. Use only hand operated roller or plate compactors within 5' of the back of walls for less than 15' high and within 10' of walls greater than 15' high.
4.7. Lay successive courses of block and layers of geogrid according to plans and profile elevations.
4.8. Notify engineer immeadiately if actual site grades/contours differ by more than 1'–0" from those indicated on these site plans. Vertical and horizontal dimensions and this specific wall(s) design is based upon approxomate site grades taken from site plans prepared by others. It is common for actual grades to differ from those as represented by contours or spot grades shown on site design plans. Variations of more than 1'–0" will effect the design of this wall. (SEE NOTE #1.1)
4.9. Owner to provide two density tests per compacted lift to ensure conformance with gradation and compaction specifications indicated. Submit final test reports for compacation testing to Associated Design Partners, Inc. for our records and also submit final test reports to the owner for their records.

PROJECT SPECIFIC DESIGN VALUES

DETAIL REFERENCE LETTER	DESIGN VALUES DESCRIPTION	E/P A/C	VALUE	UNITS	CONSTRUCTION COMPLIANCE CONFIRMED
NS	NATIVE SOIL SUBGRADE ALLOWABLE BEARING CAPACITY	E/A	3000	P.S.F.	
	NATIVE SOIL SUBGRADE INTERNAL FRICTION ANGLE	E/A	30°	DEGREES	
	NATIVE SOIL SUBGRADE UNIT WEIGHT TOTAL ±5 P.C.F.	E/A	115	P.C.F.	
	NATIVE SOIL SUBGRADE COHESION	E/A	N/A	P.C.F.	
	NATIVE RETAINED SOIL INTERNAL FRICTION ANGLE	E/A	30	DEGREES	
	NATIVE RETAINED SOIL UNIT WEIGHT TOTAL ±5 P.C.F.	E/A	130	P.C.F.	
R	REINFORCED FILL MATERIAL INTERNAL FRICTION ANGLE	P	34	DEGREES	
	REINFORCED FILL MATERIAL UNIT WEIGHT TOTAL ±5 P.C.F.	P	125	P.C.F.	
	REINFORCED FILL MATERIAL MAXIMUM PARTICLE SIZE	P	4	INCHES	
	REINFORCED FILL MATERIAL MAXIMUM FINES PASSING 200 SIEVE	P	8	PERCENT	
BF	UNREINFORCED BACKFILL MATERIAL INTERNAL FRICTION ANGLE	P	34	DEGREES	
	UNREINFORCED BACKFILL MATERIAL MAXIMUM PARTICLE SIZE	P	4	INCHES	
	UNREINFORCED BACKFILL MATERIAL MAXIMUM FINES PASSING 200 SIEVE	P	8	PERCENT	
CS	CRUSHED STONE UNIT FILL MEDIAN PARTICLE SIZE	P	3/4	INCHES	
BS	TOP OF WALL MAXIMUM BACKSLOPE ANGLE	P	18.5	DEGREES	
FB	FACE BATTER	P	NEAR VERTICAL	DEGREES	
K	SIZE OF KEYSTONE UNITS STANDARD OR COMPAC (SMALL)	P	ROCK–FACE STANDARDS	BLOCK	

LEGEND
E – EXISTING CONDITION OR VALUE
P – PROPOSED CONDITION OR VALUE
A – ASSUMED VALUE BASED UPON ANTICIPATED SITE CONDITIONS
D – DERIVED VALUE GIVEN BY OTHERS BASED UPON EXPLORATION, TESTING, OR OBSERVATION
PCF – POUNDS PER CUBIC FOOT
PSF – POUNDS PER SQUARE FOOT

REVIEWED ☒ REVIEWED & NOTED
REVIEWED SOLELY FOR GENERAL COMPLIANCE WITH CONTRACT DOCUMENTS
BLASLAND, BOUCK, & LEE, INC.
SIGNATURE
12/16/02 Date
Syr. Office location
☐ RESUBMIT ☐ REJECTED

RECEIVED
NOV 29 2002
DEC 19 2002

ASSOCIATED DESIGN PARTNERS INC.
80 Leighton Road
Falmouth, Maine 04105
Office: (207) 878-1751
Fax: (207) 878-1788
E-Mail: jbbodeau@adpengineering.com
E-Mail: dcount@adpengineering.com

PROJECT: GE PITTSFIELD MSE
PITTSFIELD, MASSACHUSETTS
FOR: KEYSTONE NORTHEAST

SHEET TITLE: KEYSTONE WALL SECTION, DESIGN VALUES CONSTRUCTION NOTES, & DETAILS

REVISIONS		
No.	DESCRIPTION	DATE
1	CHANGES PER SHOP DRAWINGS RESUBMITTAL DATED 11-19-02	11-26-02

DATE: 11–07–02
SCALE: AS NOTED
DESIGN BY: J. THIBODEAU
DRAWN BY: A. BENNETT
FILE No. 02230–RT.DWG
PROJECT NUMBER: 02230
SHEET: RT1 OF 2

PHASE 2

72 - 4" CAPS
236 - STANDARD UNITS
138 - SY OF STRATA 5200

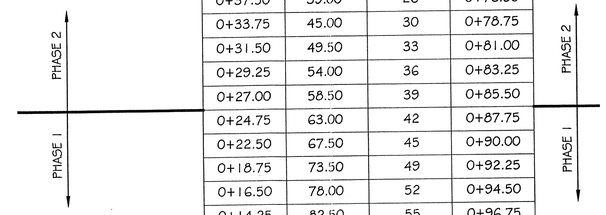
PHASE 1

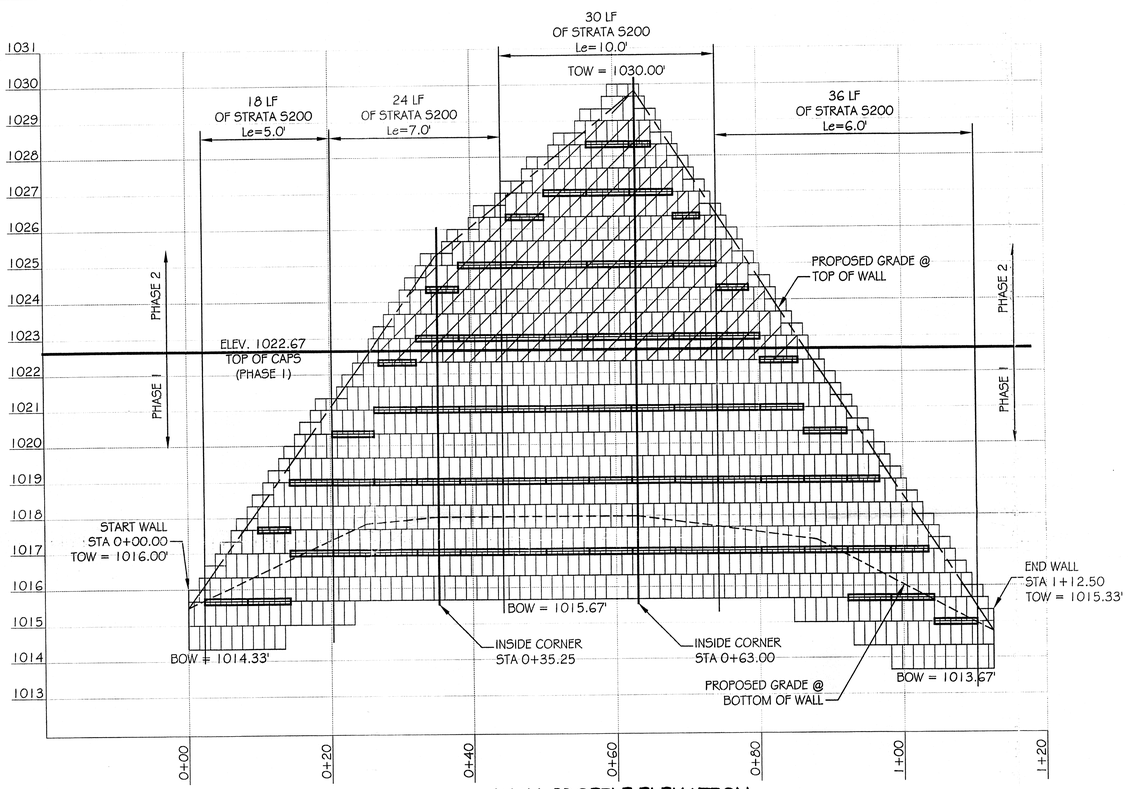
107 - 4" CAPS
627 - STANDARD UNITS
230 - SY OF STRATA 5200

START COURSE	LENGTH OF COURSE	NUMBER OF BLOCKS	END COURSE
0+00.00	112.50'	140 - 4" CAPS	1+12.50
0+60.00	3.00	2	0+63.00
0+56.25	9.00	6	0+65.25
0+52.50	15.00	10	0+67.50
0+48.75	21.00	14	0+69.75
0+45.00	27.00	18	0+72.00
0+41.25	33.00	22	0+74.25
0+37.50	39.00	26	0+76.50
0+33.75	45.00	30	0+78.75
0+31.50	49.50	33	0+81.00
0+29.25	54.00	36	0+83.25
0+27.00	58.50	39	0+85.50
0+24.75	63.00	42	0+87.75
0+22.50	67.50	45	0+90.00
0+18.75	73.50	49	0+92.25
0+16.50	78.00	52	0+94.50
0+14.25	82.50	55	0+96.75
0+12.00	87.00	58	0+99.00
0+09.75	91.50	61	1+01.25
0+07.50	96.00	64	1+03.50
0+05.25	100.50	67	1+05.75
0+03.00	105.00	70	1+08.00
0+00.00 / 0+84.75	23.25 / 25.50	15.5 / 17	0+23.25 / 1+10.25
0+00.00 / 0+93.00	13.50 / 19.50	9 / 13	0+13.50 / 1+12.50
0+98.25	14.25	9.5	1+12.50

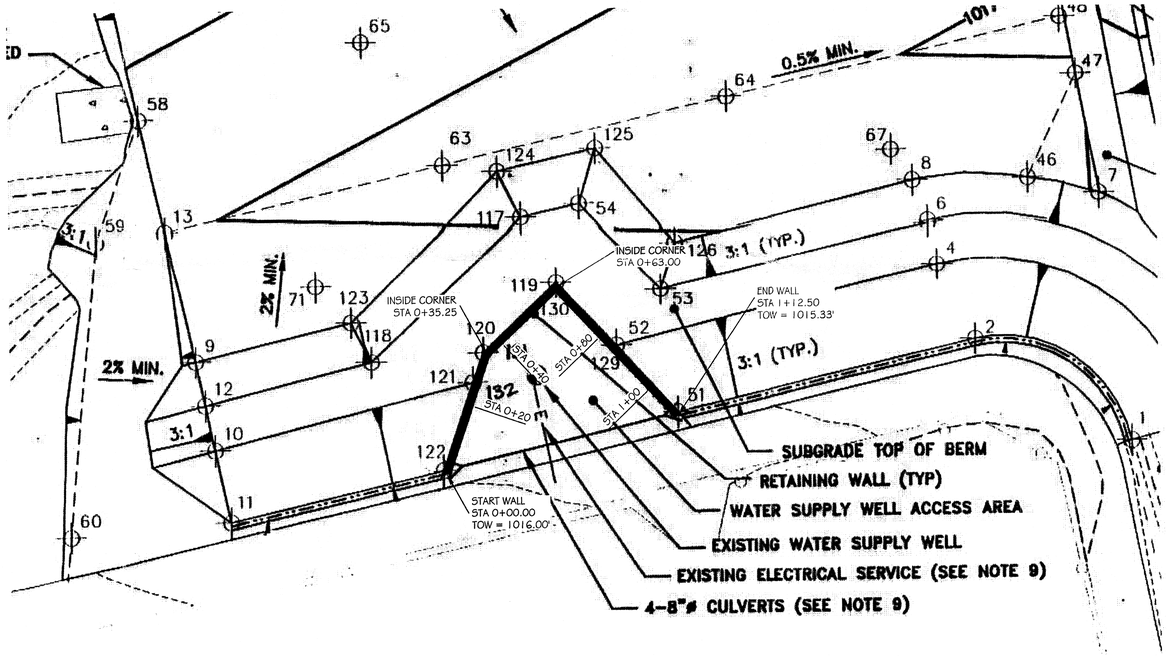
WALL TOTALS

140 - 4" CAPS
863 - STANDARD ROCK-FACE UNITS
368 - SY OF STRATA 5200





WALL PROFILE ELEVATION
SCALE: 1"-2'-0" VERT
SCALE: 1"-10'-0" HORZ



PARTIAL SITE PLAN
SCALE: 1"-20'-0"

ASSOCIATED DESIGN PARTNERS INC.
80 Leighton Road
Falmouth, Maine 04105
Office: (207) 878-1751
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PROJECT: GE PITTSFIELD MSE
PITTSFIELD, MASSACHUSETTS
FOR: KEYSTONE NORTHEAST

SHEET TITLE: PARTIAL SITE PLAN & WALL ELEVATION PROFILE

No.	DESCRIPTION	DATE
1	CHANGES PER SHOP DRAWINGS RESUBMITTAL DATED 11-19-02	11-26-02

REVISIONS

BLASLAND, BOUCK & LEE, INC.
SIGNATURE
12/16/02 Date
Syr. Office Location

DATE: 11-07-02
SCALE: AS NOTED
DESIGN BY: J. THIBODEAU
DRAWN BY: A. BENNETT
FILE No. 02230-RT.DWG
PROJECT NUMBER: 02230
SHEET: RT2 OF 2

RECEIVED DEC 19
RECEIVED NOV 29 2002